

APPENDIX 2: TRAFFIC ANALYSIS

Introduction

Recommendations to reuse excess roadbed for new bicycle lanes along the five-mile study corridor do not remove travel lanes. Excessively wide streets afford the opportunity to install on-street bicycle facilities without affecting traffic operations, and no traffic impact analysis is required. However, existing and future traffic conditions were assessed and vehicular levels-of-service (LOS) were calculated in order to better understand route conditions for cyclists and pedestrians. Data was collected and analyzed at seven major intersections along the study corridor:

- Link 1: Shore Parkway Off-ramp/Shore Road South and Bay Parkway;
- Link 3: Neptune Avenue and Cropsey Avenue/West 17th Street;
- Link 4: Neptune Avenue and West 31st Street;
- Link 5: Neptune Avenue and Stillwell Avenue;
- Link 5: Neptune Avenue and Ocean Parkway;
- Link 6: Neptune Avenue and Coney Island Avenue; and
- Link 7: Emmons Avenue and Nostrand Avenue.

Future no-build and build traffic conditions were then identified and analyzed based on a build year of 2005 for the striping of Class 2 bicycle lanes on-street.

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Highway Capacity Manual and Software

The operation of signalized intersections within the study area was analyzed applying the methodologies presented in the 2000 Highway Capacity Manual (HCM2000). These procedures evaluate signalized intersections for average delay per vehicle and level of service (LOS). The capacity analysis methodology separates an intersection approach into lane groups on the basis of the movements occurring during each signal phase. The lane groups are then analyzed to determine the specific vehicular capacity and LOS. This analysis requires the following input parameters: intersection geometry, lane utilization, number and width of travel lanes, on-street parking conditions, locations of bus stops, number of buses stopping per hour, vehicle turning movements, vehicle classification, conflicting pedestrian movements, traffic signal cycle length, and allocation of green time.

The operating characteristics of signalized intersections can be estimated and evaluated by analyzing capacity and performance. The capacity of an intersection represents the throughput of a facility (i.e., the maximum number of vehicles that can be processed in one hour). Capacity analysis results in the volume-to-capacity ratio (v/c ratio) which presents the proportion of capacity (supply) utilized by the existing traffic volume (demand). High v/c ratios (>0.85) indicate some traffic congestion, and low v/c ratios (<0.60) indicate a smooth traffic flow.

The performance of an intersection is based on the estimated average delay time (i.e., the average stopped time per vehicle) for each vehicle utilizing a roadway segment. Delay time is determined by the capacity of a lane group, the amount of green time allotted to a lane group, and the signal cycle length. Delay time is the factor which determines the LOS for a lane group. Short delays correspond to a good LOS while long delays correspond to a poor LOS. For example, an average delay of up to ten seconds per vehicle is categorized as LOS A, and 80 seconds delay is categorized as LOS F. In New York City, an LOS of mid-D, corresponding to average delay of 45 second, is considered acceptable. Table 1 describes the LOS definitions for signalized intersections.

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Table 1: Level of Service Definitions for Signalized Intersections

Flow Quality	Description
Level A	Describes operation with very low delay, i.e., less than or equal to 10 seconds per vehicle. This occurs when progression is extremely favorable and most vehicles arrive during the green phase. Most vehicles do not stop at all. Short cycle lengths may also contribute to low delay.
Level B	Describes operation with delay in the range of >10-20 seconds per vehicle. This generally occurs with good progression and/or short cycle lengths. More vehicles stop than for LOS A, causing higher levels of average delay.
Level C	Describes operation with delay in the range of >20-35 seconds per vehicle. These higher delays may result from fair progression and/or longer cycle lengths. Individual cycle failures may begin to appear at this level. The number of vehicles stopping is significant at this level, although some may still pass through the intersection without stopping.
Level D	Describes operation with delay in the range of >35-55 seconds per vehicle. At level D, the influence of congestion becomes more noticeable. Longer delays may result from some combination of unfavorable progression, longer cycle lengths, or high v/c ratios. Many vehicles stop and the proportion of vehicles not stopping declines. Individual cycle failures are noticeable.
Level E	Describes operation with delay in the range of >55-80 seconds per vehicle. This is considered to be the limit of acceptable delay. These high delay values generally indicate poor progression, long cycle lengths, and high v/c ratios. Individual cycle failures are frequent occurrences.
Level F	Describes operation with delay in excess of 80.0 seconds per vehicle. This is considered to be unacceptable to most drivers. This condition often occurs with saturation, i.e., when arrival flow rates exceed the capacity of the intersection. It may also occur at high v/c ratios below 1.00 with many individual cycle failures. Poor progression and long cycle lengths may also be major contributing causes to such delay levels.
Source:	<i>Highway Capacity Manual</i> , Transportation Research Board, National Research Council, Washington, D.C., 2000

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Data Collection

As mentioned, a traffic analysis was performed to assure the feasibility of recommendations to narrow the roadway on Shore Parkway South and on Neptune and Emmons avenues to accommodate new pedestrian and bicycle facilities. The studies involved extensive field work, route reconnaissance, and data collection, including automatic and manual traffic volume and vehicle classification counts.

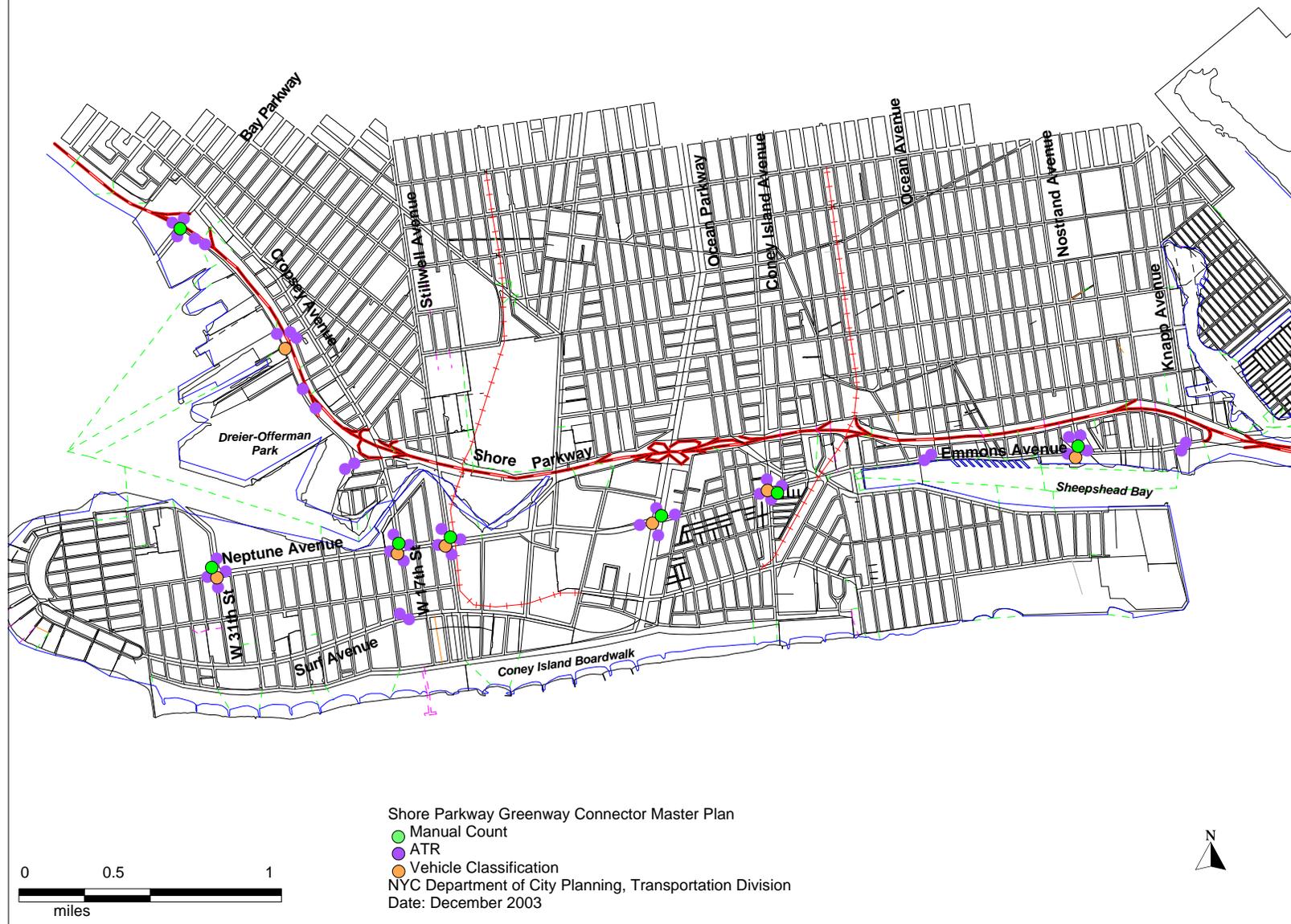
Vehicular Traffic

Forty-two automatic traffic recorders (ATRs) were setup at various locations along the corridor for a two-week period in July 2003. Data was collected in the summer, a period typically considered abnormal for traffic studies, in order to capture the peak seasonal traffic generated by activities on the Coney Island waterfront.

A preliminary AM, MD, PM and weekend peak hour in the corridor was identified using available study area ATRs from the past three years. Then two-hour manual counts were conducted at the seven selected intersections for the AM (7:30-9:30 am), MD (12:00-2:00 pm), PM (4:30-6:30 pm), and weekend (12:00-2:00 pm) peak period during the same period in July when ATR data was collected. Twenty-minute (or one-hundred-vehicle minimum) vehicle classification counts were also conducted at these seven intersections. Map 1 shows automatic and manual count and vehicle classification locations. (Vehicles entering or exiting the mall to/from Shore Road South were counted at three parking lot driveways.)

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Map 1. ATR, Manual Count and Vehicle Classification Count Locations, August 2003



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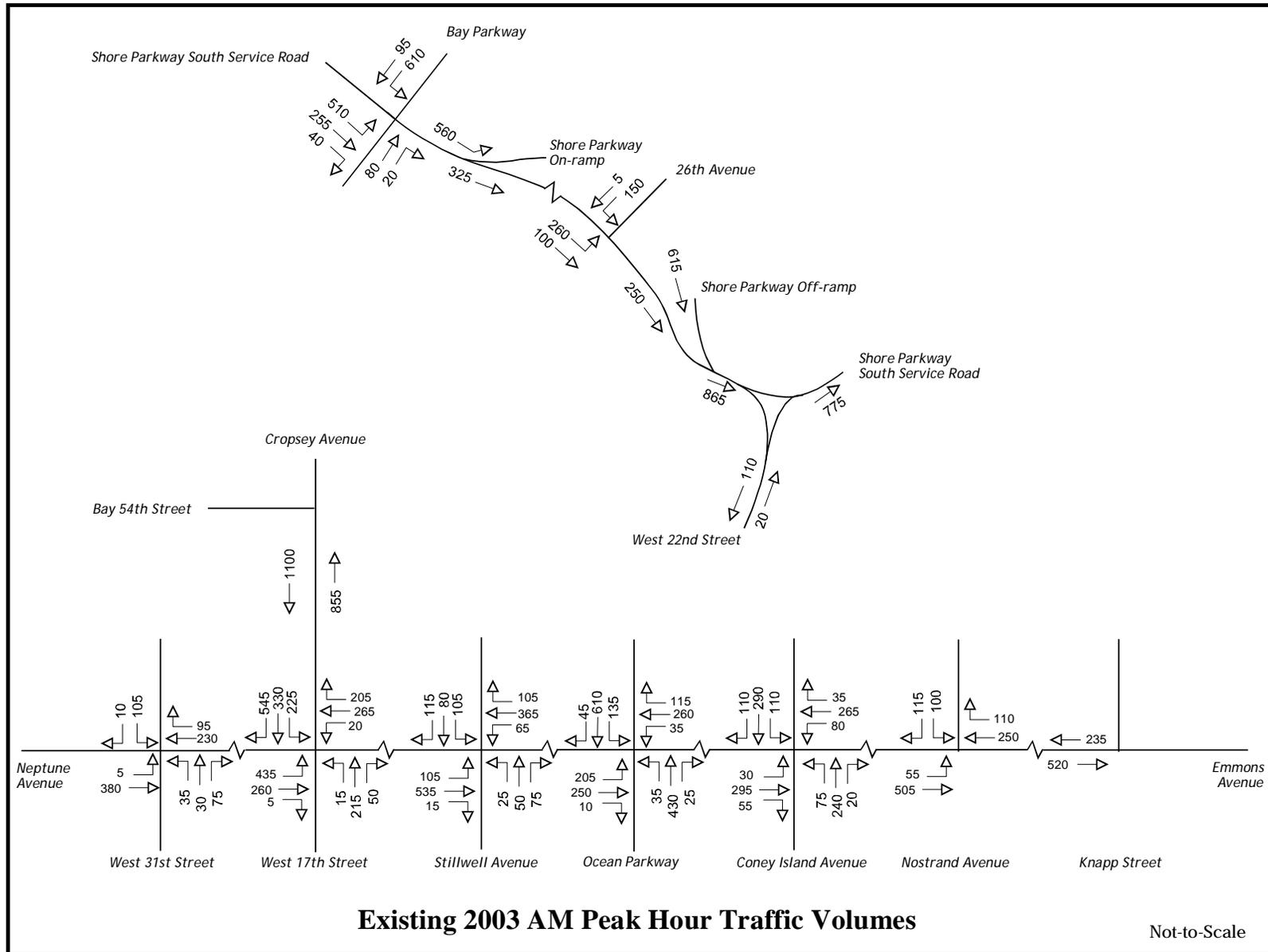
Existing Conditions

Typical weekday and weekend peak hour traffic volumes were compiled for the AM (8:15-9:15 am), MD (1:00-2:00 pm), PM (5:00-6:00 pm), and weekend (Saturday 1:00-2:00 pm) from old and new ATRs, manual counts, and axle factor data. The balanced existing traffic volumes for each period are shown in Figures 1, 2, 3 and 4, respectively. Traffic signal phasing and timing was requested from the City DOT, verified in the field, and reconfirmed with DOT. Bus and parking movements were studied and recorded, and, at key intersections, pedestrians and bicyclists were counted.

Table 2 presents a summary of the existing LOS at the seven intersections. Cropsey and Neptune avenues experience the worst overall traffic conditions, especially the southbound left turn movement. East- west traffic on Neptune Avenue at Stillwell Avenue has a low level of service, perhaps due to the allocation of more green time (76 seconds) to the north- south traffic at the expense of signal time (34 seconds) for the east-west traffic. Eastbound left turns on Neptune Avenue at Ocean Parkway operate poorly at LOS E. During the morning peak hour, the eastbound left and through movements on Emmons and Neptune avenues operate at LOS E due to traffic bound for the Shore Parkway.

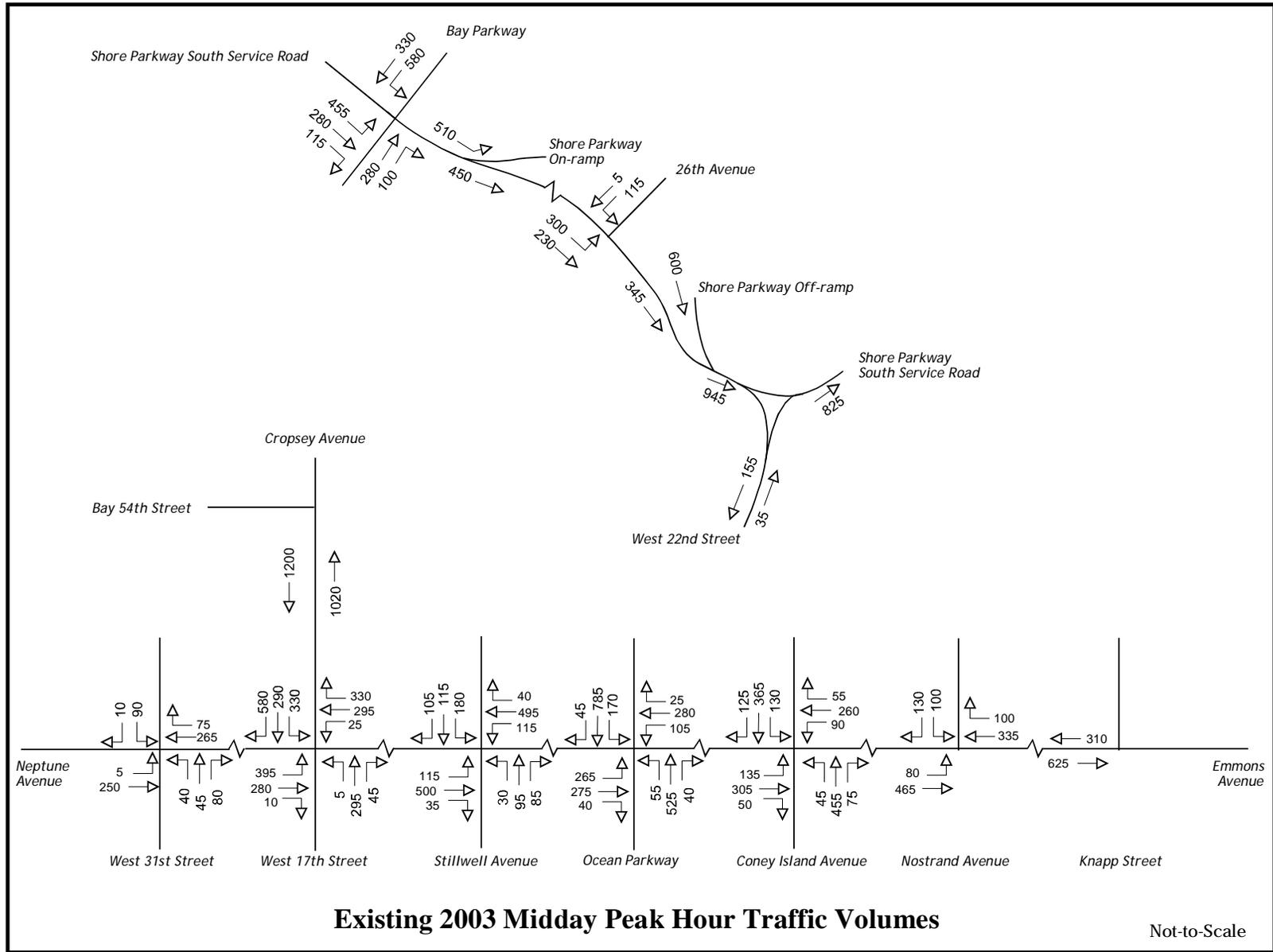
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Figure 1



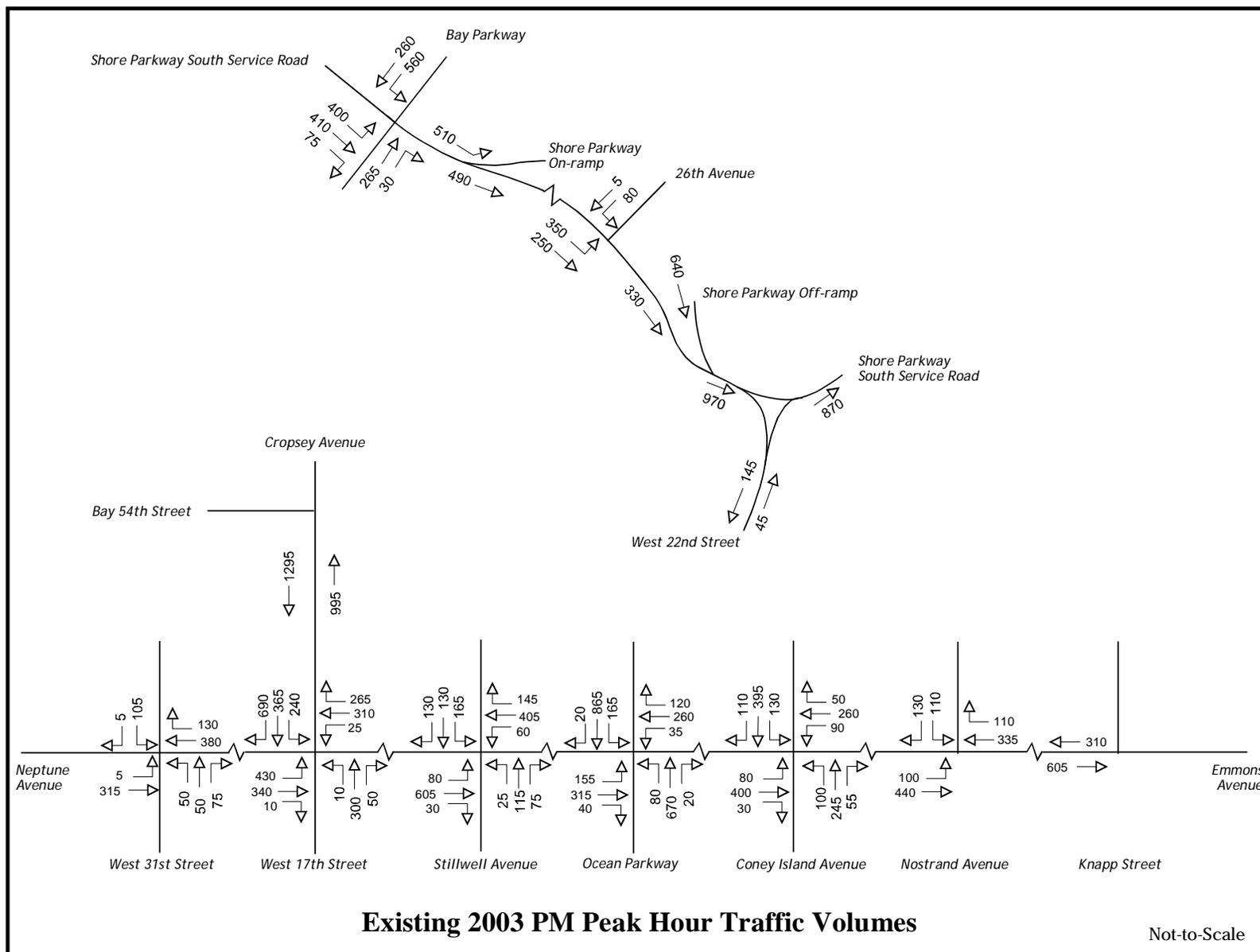
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Figure 2



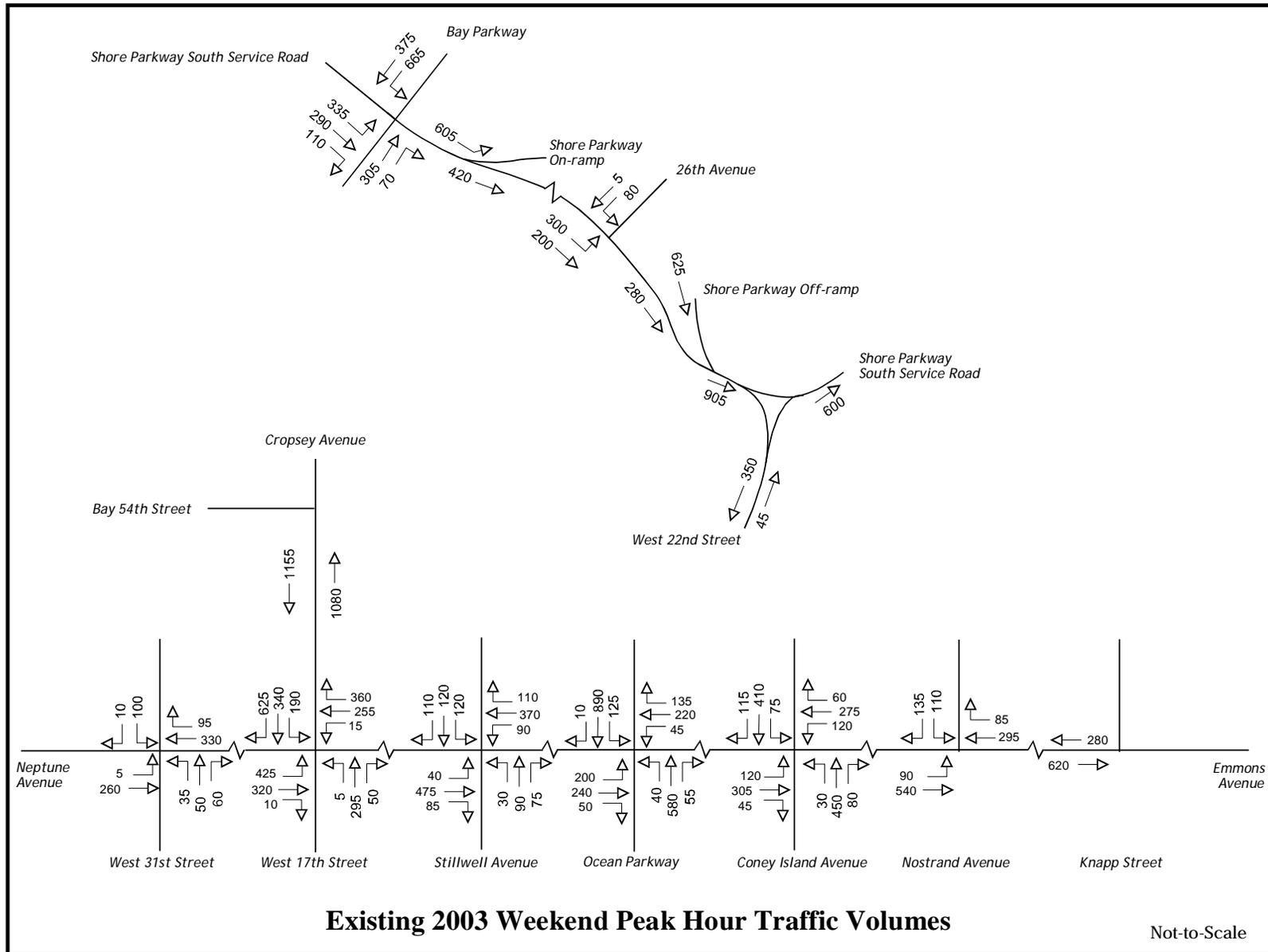
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Figure 3



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Figure 4



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Table 2

2003 Existing Traffic Conditions														
Signalized Intersections	Lane Group	Weekday									Weekend			
		AM (7:15am-8:15am)			MD (1:00pm-2:00pm)			PM (5:00pm-6:00pm)			MD (1:00pm-2:00pm)			
		V/C Ratio	Delay (sec/veh)	LOS										
Shore Pkwy Service Rd (E) Bay Parkway (N-S)	EB-L	1.00	79.9	E	0.88	46.6	D	0.80	49.5	D	0.63	31.3	C	
	EB-TR	0.59	39.2	D	0.80	39.9	D	0.99	77.9	E	0.78	38.7	D	
	NB-T	0.13	32.2	C	0.37	28.1	C	0.33	35.1	D	0.40	28.6	C	
	NB-R	0.07	31.9	C	0.33	29.2	C	0.09	32.2	C	0.23	27.5	C	
	SB-L	0.26	23.6	C	0.33	23.8	C	0.23	24.1	C	0.43	27.2	C	
	SB-LT	0.40	38.4	D	0.84	45.2	D	0.60	42.5	D	0.90	52.0	D	
Neptune Avenue (E-W) West 31 Street (N-S)	EB-LT	0.29	11.7	B	0.17	10.8	B	0.23	11.2	B	0.19	10.9	B	
	WB-TR	0.17	10.7	B	0.19	10.9	B	0.27	11.6	B	0.23	11.3	B	
	NB-LTR	0.37	24.7	C	0.42	25.7	C	0.4	25.3	C	0.34	24.2	C	
	SB-L	0.31	24.1	C	0.32	24.4	C	0.34	24.6	C	0.26	23	C	
	SB-R	0.03	19.9	B	0.03	19.9	B	0.01	19.8	B	0.02	19.8	B	
	Neptune Avenue (E-W) Cropsey Avenue (N-S)	EB-L	0.61	15.5	B	0.51	14.5	B	0.61	15.8	B	0.53	14.9	B
EB-TR	0.17	10.2	B	0.18	10.3	B	0.23	10.8	B	0.20	10.5	B		
WB-L	0.09	22.2	C	0.12	22.7	C	0.13	23.0	C	0.07	21.9	C		
WB-TR	0.50	26.6	C	0.68	30.5	C	0.62	28.7	C	0.66	29.7	C		
NB-LTR	0.74	38.5	D	0.94	61.4	E	1.00	75.1	E	0.85	45.8	D		
SB-L	0.91	66.8	E	0.98	73.1	E	0.97	78.1	E	0.92	72.9	E		
SB-T	0.61	30.1	C	0.56	28.9	C	0.68	32.5	C	0.61	30.2	C		
SB-R	0.44	12.6	B	0.49	13.4	B	0.56	14.4	B	0.49	13.3	B		
Neptune Avenue (E-W) Stillwell Avenue (N-S)	EB-LTR(DEF L) (EB-TR)	0.52	19.8	B	0.54	27.6	C	0.54	20	C	0.39	17.8	B	
	WB-LTR	0.66	23.8	C	0.83	32.0	C	0.73	26.1	C	0.67	24.2	C	
	NB-LTR	0.18	15.8	B	0.22	16.1	B	0.25	16.5	B	0.19	15.8	B	
	SB-LTR(DEF L) (SB-TR)	0.43	19.1	B	0.44	20.9	C	0.53	20.9	C	0.43	18.9	B	
	EB-L	0.84	62.3	E	0.93	76.8	E	0.53	31.2	C	0.67	42.8	D	
	EB-T	0.45	28.8	C	0.43	27.0	C	0.50	29.5	C	0.37	25.7	C	
Neptune Avenue (E-W) Ocean Parkway (N-S)	EB-R	0.02	21.9	C	0.08	21.4	C	0.08	22.6	C	0.09	21.6	C	
	WB-LTR	0.57	39.6	D	0.61	39.2	D	0.55	39.3	D	0.50	36.0	D	
	NB-L	0.10	21.1	C	0.17	26.5	C	0.27	31.8	C	0.13	26.8	C	
	NB-TR	0.34	29.5	C	0.44	32.3	C	0.56	34.5	C	0.49	33.1	C	
	SB-L	0.34	23.1	C	0.45	30.3	C	0.51	36.5	D	0.34	27.5	C	
	SB-TR	0.54	32.8	C	0.66	36.8	D	0.77	40.5	D	0.71	38.1	D	
	EB-L	0.23	36.6	D	0.48	25.2	C	0.61	55.6	E	0.46	24.7	C	
	EB-TR	0.93	71.2	E	0.62	25.8	C	0.98	77.9	E	0.62	25.6	C	
Neptune Avenue (E-W) Coney Island Avenue (N-S)	WB-L	0.76	76.9	E	0.37	22.5	C	0.79	77.3	E	0.48	25.4	C	
	WB-TR	0.78	52.7	D	0.56	24.3	C	0.83	57.2	E	0.57	24.1	C	
	NB-LTR	0.26	10.1	B	0.49	17.5	B	0.34	11.0	B	0.45	16.8	B	
	SB-LTR	0.39	11.5	B	0.68	22.1	C	0.46	12.5	B	0.55	18.6	B	
	Emmons Avenue (E-W) Nostrand Avenue (N-S)	EB-LT	1.00	69.4	E	0.51	19.6	B	0.57	26.1	C	0.56	20.5	C
	WB-TR	0.29	20.9	C	0.34	17.1	B	0.35	21.6	C	0.32	16.8	B	
SB-L	0.19	19.9	B	0.16	15.4	B	0.16	19.5	B	0.16	15.4	B		
SB-R	0.25	21.0	C	0.24	16.5	B	0.23	20.5	C	0.23	16.3	B		

Shaded area indicates "E" level-of-service.

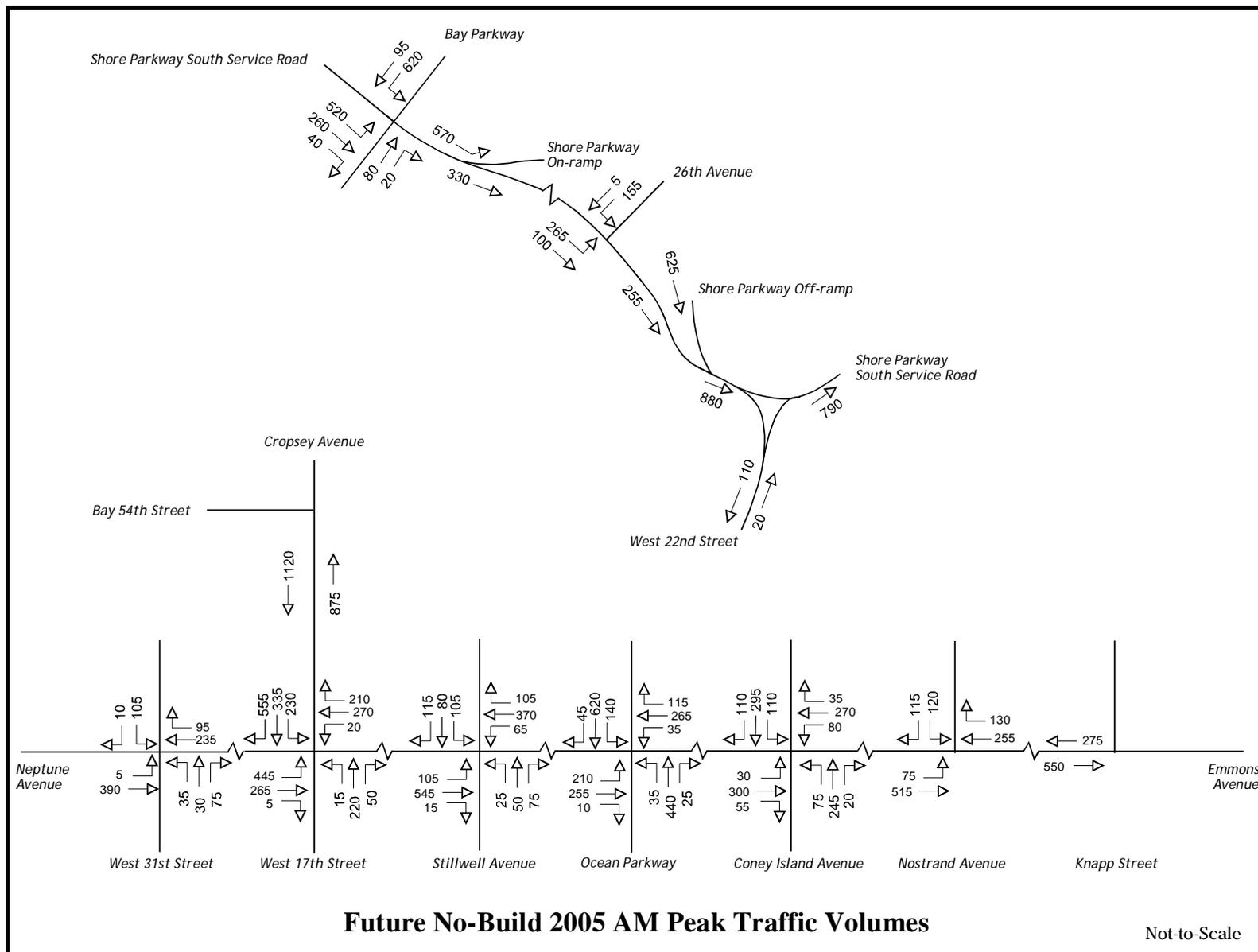
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Future No-Build Conditions

Based on the existing traffic volumes, 2005 future no-build traffic volumes were constructed. A background one percent annual growth rate, as specified for Brooklyn in the updated City Environmental Quality Review (CEQR) Technical Manual, was applied to the existing traffic volume from 2003 to 2004. At year 2004, a 125-unit residential development on Emmons Avenue is planned. Using standard trip generation and assignment methodologies, the number of project-generated trips was calculated and added to the street network. Then another one percent annual growth rate was applied to the 2004 network with development to determine 2005 future traffic volumes for each intersection for each peak hour as shown in Figures 5, 6, 7, and 8, respectively.

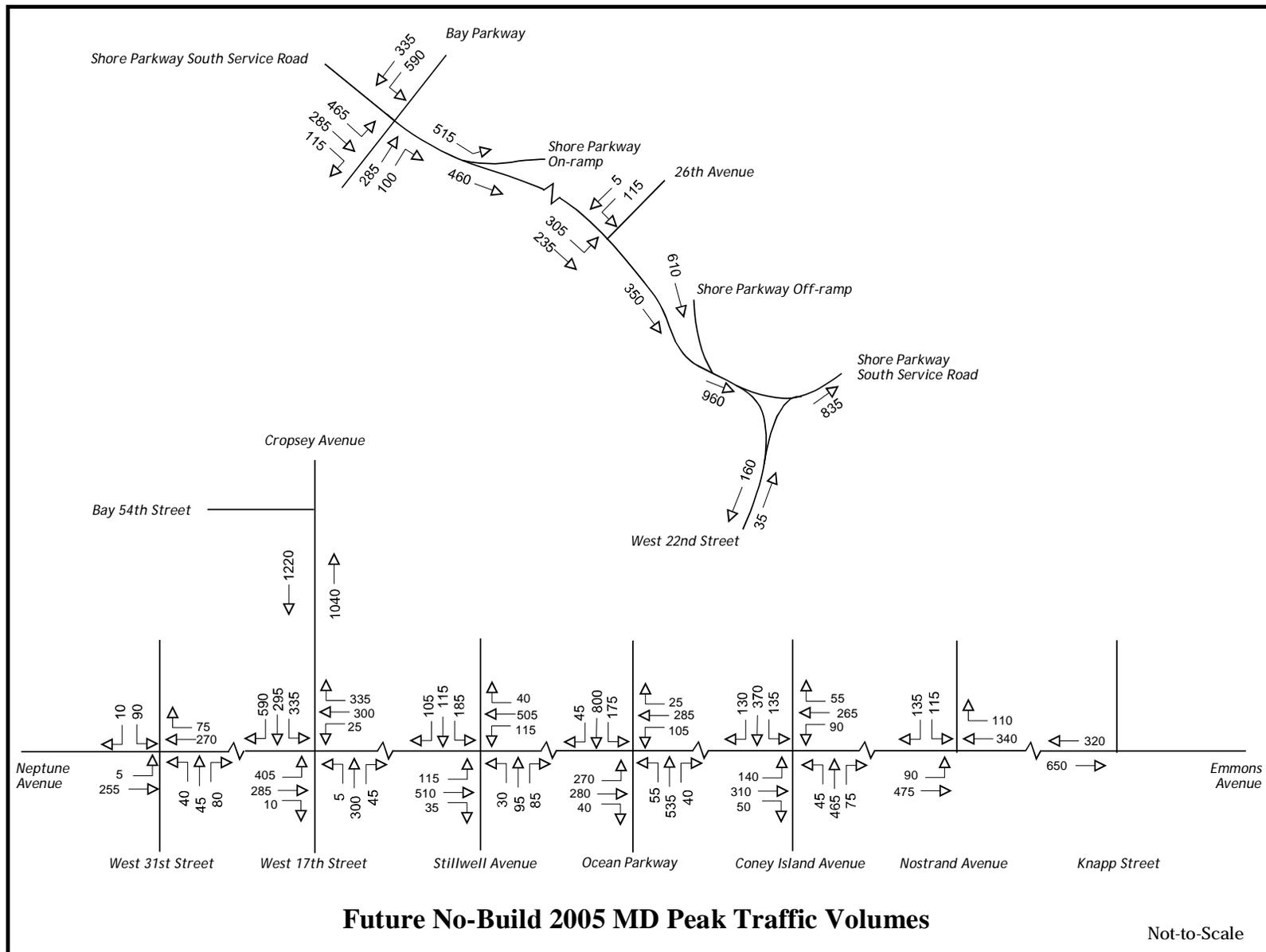
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Figure 5



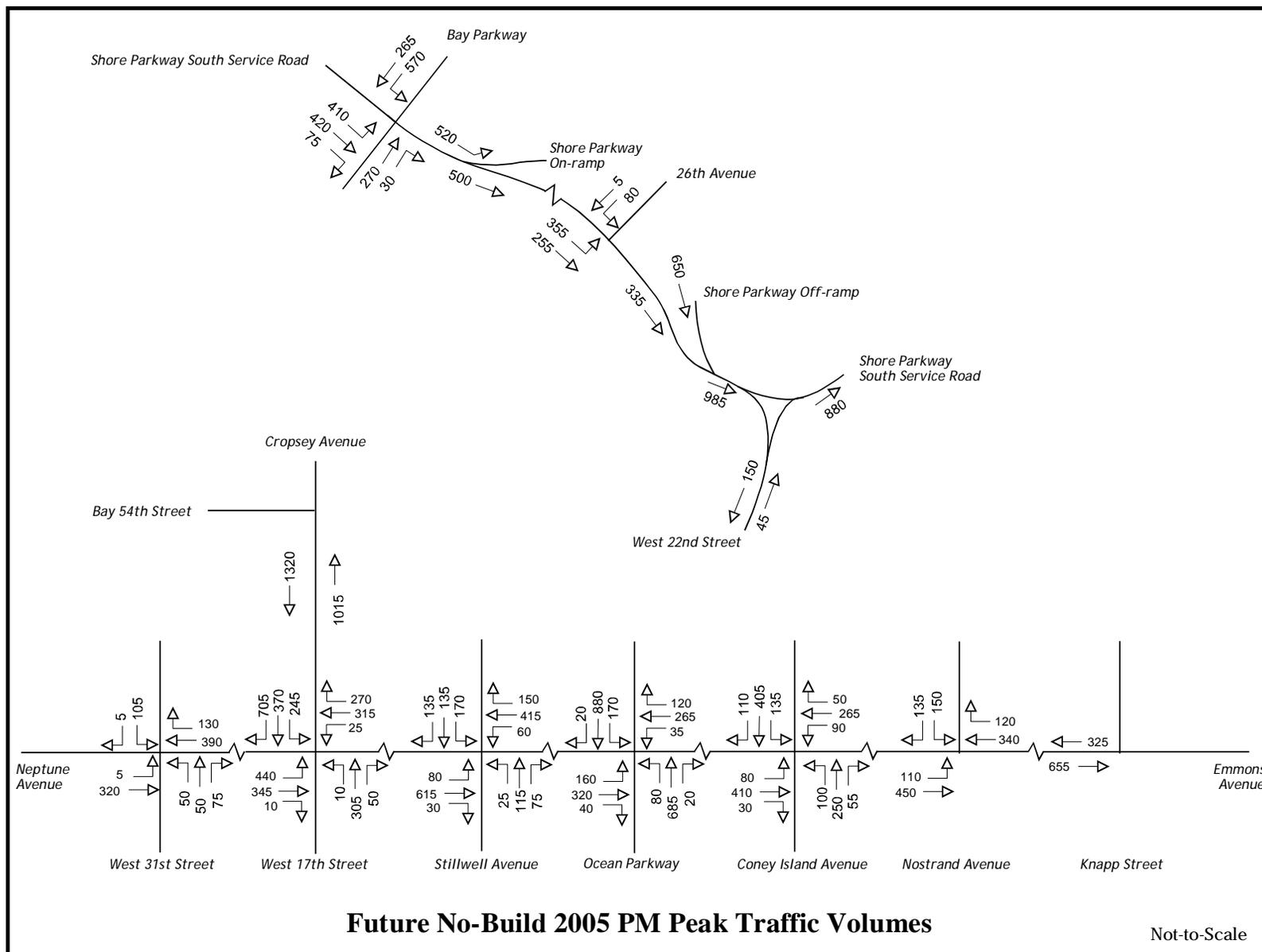
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Figure 6



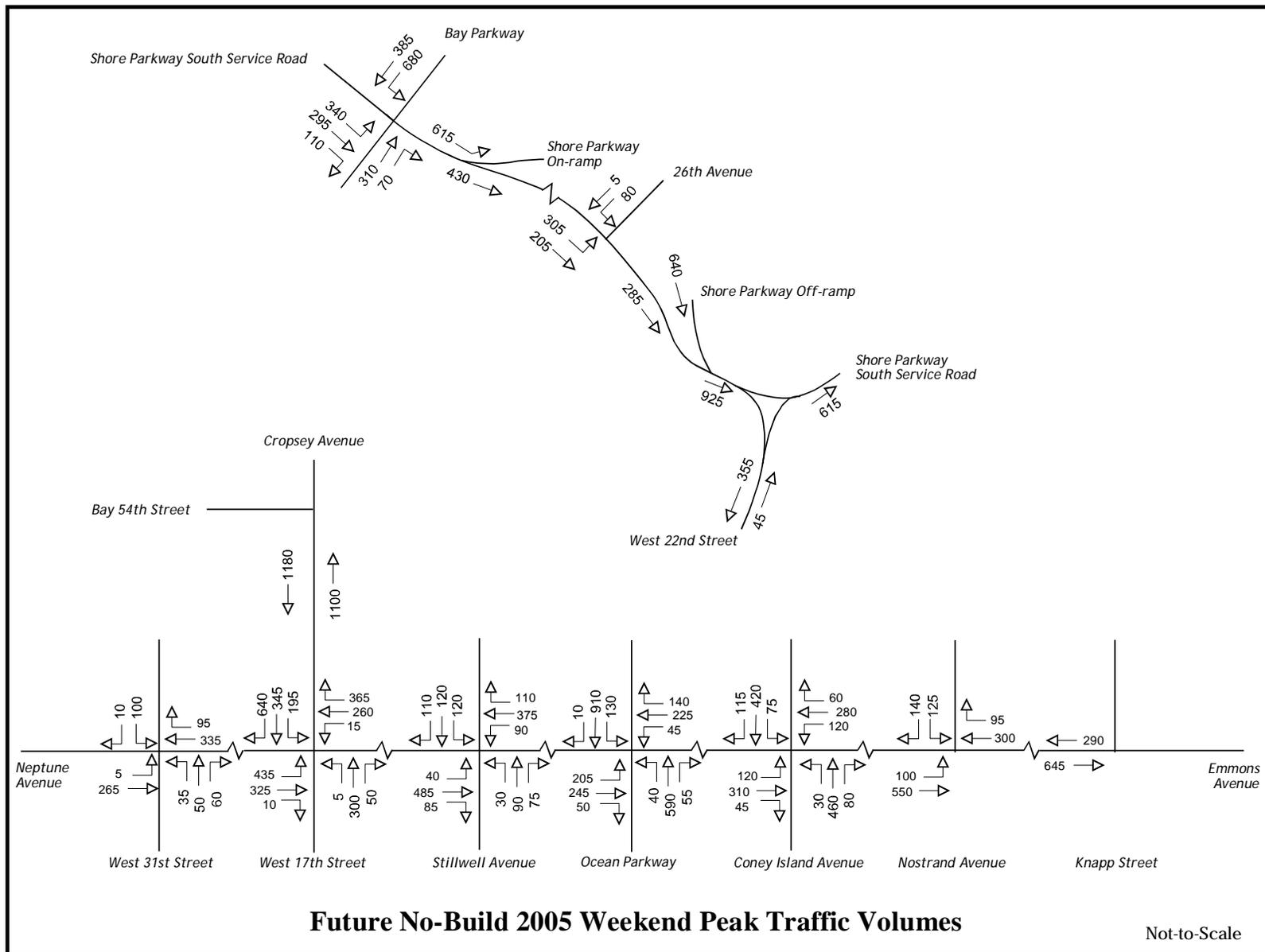
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Figure 7



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Figure 8



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Table 3

Signalized Intersections		Lane Group		Weekday									Weekend		
				AM (7:15am-8:15am)			MD (1:00pm-2:00pm)			PM (5:00pm-6:00pm)			MD (1:00pm-2:00pm)		
				V/C Ratio	Delay (sec/veh)	LOS	V/C Ratio	Delay (sec/veh)	LOS	V/C Ratio	Delay (sec/veh)	LOS	V/C Ratio	Delay (sec/veh)	LOS
Shore Pkwy Service Rd (E) Bay Parkway (N-S)	EB-L	1.02	84.9	F	0.90	49.1	D	0.82	51.2	D	0.64	31.6	C		
	EB-TR	0.60	39.6	E	0.81	40.6	D	1.01	83.0	F	0.79	39.3	D		
	NB-T	0.13	32.2	C	0.37	28.2	C	0.34	35.2	D	0.41	28.6	C		
	NB-R	0.08	31.9	C	0.33	29.2	C	0.09	32.2	C	0.23	27.5	C		
	SB-L	0.28	24.1	C	0.36	24.4	C	0.25	24.7	C	0.51	28.2	C		
	SB-LT	0.41	38.5	D	0.85	46.3	D	0.61	42.8	D	0.93	55.2	E		
Neptune Avenue (E-W)	EB-LT	0.3	11.8	B	0.18	10.8	B	0.23	11.2	B	0.19	10.9	B		
West 31 Street (N-S)	WB-TR	0.17	10.8	B	0.2	10.9	B	0.28	11.7	B	0.24	11.3	B		
	NB-LTR	0.37	24.7	C	0.42	25.7	C	0.4	25.3	C	0.34	24.2	C		
	SB-L	0.31	24.1	C	0.32	24.4	C	0.34	24.6	C	0.26	23	C		
	SB-R	0.03	19.9	B	0.03	19.9	B	0.01	19.8	B	0.02	19.8	B		
Neptune Avenue (E-W)	EB-L	0.62	15.7	B	0.52	14.7	B	0.62	16.1	B	0.54	15.1	B		
Cropsey Avenue (N-S)	EB-TR	0.17	10.3	B	0.18	10.3	B	0.23	10.8	B	0.20	10.5	B		
	WB-L	0.09	22.2	C	0.12	22.7	C	0.13	23.0	C	0.07	21.9	C		
	WB-TR	0.51	26.8	C	0.69	30.8	C	0.63	29.0	C	0.67	30.0	C		
	NB-LTR	0.75	39.4	D	0.96	64.4	E	1.02	78.8	E	0.86	47.1	D		
	SB-L	0.93	70.7	E	0.99	76.7	E	0.99	82.8	F	0.94	77.9	E		
	SB-T	0.61	30.3	C	0.57	29.2	C	0.69	32.8	C	0.62	30.5	C		
	SB-R	0.44	12.7	B	0.50	13.5	B	0.57	14.6	B	0.50	13.5	B		
Neptune Avenue (E-W)	EB-LTR(DEF L)	0.53	19.9	B	0.55	28.1	C	0.55	20.2	C	0.4	17.9	B		
Stillwell Avenue (N-S)	(EB-TR)				0.48	19.2	B								
	WB-LTR	0.67	24	C	0.85	33.1	C	0.75	26.8	C	0.68	24.4	C		
	NB-LTR	0.18	15.8	B	0.22	16.1	B	0.25	16.5	B	0.19	15.8	B		
	SB-LTR(DEF L)	0.43	19.1	B	0.45	21.1	C	0.55	21.3	C	0.43	18.9	B		
	(SB-TR)				0.46	20.8	C								
Neptune Avenue (E-W)	EB-L	0.86	66.2	E	0.96	82.3	F	0.55	31.8	C	0.69	44.9	D		
Ocean Parkway (N-S)	EB-T	0.46	29.0	C	0.44	27.2	C	0.50	29.7	C	0.38	25.8	C		
	EB-R	0.02	21.9	C	0.08	21.4	C	0.08	22.6	C	0.09	21.6	C		
	WB-LTR	0.57	39.8	D	0.62	39.4	D	0.56	39.5	D	0.51	36.3	D		
	NB-L	0.10	21.3	C	0.17	26.9	C	0.27	32.3	C	0.13	27.2	C		
	NB-TR	0.35	29.6	C	0.45	32.4	C	0.58	34.7	C	0.50	33.3	C		
	SB-L	0.36	23.7	C	0.47	31.1	C	0.53	37.7	D	0.36	28.2	C		
	SB-TR	0.55	33.0	C	0.67	37.1	D	0.79	41.0	D	0.73	38.6	D		
Neptune Avenue (E-W)	EB-L	0.23	36.8	D	0.51	26.0	C	0.61	55.6	E	0.47	25.0	C		
Coney Island Avenue (N-S)	EB-TR	0.94	74.0	E	0.63	26.0	C	1.00	83.8	F	0.63	25.9	C		
	WB-L	0.79	82.3	F	0.37	22.6	C	0.79	77.3	E	0.48	25.7	C		
	WB-TR	0.79	53.6	D	0.57	24.5	C	0.84	58.5	E	0.58	24.3	C		
	NB-LTR	0.27	10.2	B	0.50	17.6	B	0.34	11.1	B	0.46	16.9	B		
	SB-LTR	0.39	11.5	B	0.71	23.0	C	0.48	12.8	B	0.56	18.8	B		
Emmons Avenue (E-W)	EB-LT	1.16	123	F	0.54	20.2	C	0.61	27.2	C	0.6	21.2	C		
Nostrand Avenue (N-S)	WB-TR	0.31	21.2	C	0.36	17.2	B	0.36	21.8	C	0.33	16.9	B		
	SB-L	0.23	20.5	C	0.18	15.7	B	0.22	20.3	C	0.18	15.7	B		
	SB-R	0.25	21	C	0.24	16.6	B	0.23	20.6	C	0.23	16.4	B		

Shaded area indicates an "E" or "F" level of service.

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Future Build Conditions

HCS LOS analysis was conducted using the 2005 future no-build traffic volumes, as summarized in Table 3. LOS deteriorated during some or all peak periods, requiring mitigation at five intersections:

1. Shore Parkway Off-ramp/South Service Road and Bay Parkway in the AM, PM, and weekend peak;
2. Neptune Avenue and Cropsey avenues during all peak periods;
3. Neptune Avenue and Ocean Parkway in the AM and Midday peak;
4. Neptune Avenue and Coney Island Avenue in the AM and PM peak; and
5. Emmons and Nostrand avenues in the AM peak period.

Signal timing changes, recommended to address the poor vehicular LOS, improve the performance of each analyzed intersection as summarized in Table 4. Improved traffic conditions would benefit bicyclists and pedestrians by reducing vehicular congestion and facilitating turning movements. Table 5 shows a comparison of the existing and proposed signal timing at each of the five intersections.

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Table 4

2005 Future No-Build Conditions with Recommended Signal Timing Changes													
Signalized Intersections	Lane Group	Weekday									Weekend		
		AM (7:15am-8:15am)			MD (1:00pm-2:00pm)			PM (5:00pm-6:00pm)			MD (1:00pm-2:00pm)		
		V/C Ratio	Delay (sec/veh)	LOS									
Shore Pkwy Service Rd (E) Bay Parkway (N-S)	EB-L	0.90	55.5	E	0.90	49.1	D	0.73	40.8	D	0.64	31.6	C
	EB-TR	0.53	33.8	C	0.81	40.6	D	0.89	54.6	D	0.79	39.3	D
	NB-T	0.14	34.6	C	0.37	28.2	C	0.37	37.9	D	0.44	30.8	C
	NB-R	0.09	34.2	C	0.33	29.2	C	0.10	34.6	C	0.25	29.5	C
	SB-L	0.38	29.2	C	0.36	24.4	C	0.35	29.9	C	0.54	28.0	C
	SB-LT	0.44	40.5	D	0.85	46.3	D	0.66	45.6	D	0.84	43.7	D
Neptune Avenue (E-W) West 31 Street (N-S)	EB-LT	0.3	11.8	B	0.18	10.8	B	0.23	11.2	B	0.19	10.9	B
	WB-TR	0.17	10.8	B	0.2	10.9	B	0.28	11.7	B	0.24	11.3	B
	NB-LTR	0.37	24.7	C	0.42	25.7	C	0.4	25.3	C	0.34	24.2	C
	SB-L	0.31	24.1	C	0.32	24.4	C	0.34	24.6	C	0.26	23	C
	SB-R	0.03	19.9	B	0.03	19.9	B	0.01	19.8	B	0.02	19.8	B
Neptune Avenue (E-W) Cropsey Avenue (N-S)	EB-L	0.62	17.9	B	0.51	16.8	B	0.61	18.2	B	0.53	17.3	B
	EB-TR	0.18	12.4	B	0.20	12.5	B	0.26	13.0	B	0.22	12.7	B
	WB-L	0.11	25.4	C	0.14	26.0	C	0.15	26.4	C	0.08	25.0	C
	WB-TR	0.59	31.3	C	0.80	38.4	D	0.73	34.9	C	0.78	36.9	D
	NB-LTR	0.66	30.9	C	0.84	42.7	D	0.90	48.8	D	0.76	34.8	C
	SB-L	0.82	48.1	D	0.88	49.0	D	0.87	54.0	D	0.83	53.1	D
	SB-T	0.54	25.4	C	0.50	24.6	C	0.60	27.0	C	0.55	25.5	C
	SB-R	0.41	10.3	B	0.46	10.9	B	0.53	11.8	B	0.47	10.9	B
	EB-LTR(DEF L)	0.53	19.9	B	0.55	28.1	C	0.55	20.2	C	0.4	17.9	B
Neptune Avenue (E-W) Stillwell Avenue (N-S)	EB-TR				0.48	19.2	B						
	WB-LTR	0.67	24	C	0.85	33.1	C	0.75	26.8	C	0.68	24.4	C
	NB-LTR	0.18	15.8	B	0.22	16.1	B	0.25	16.5	B	0.19	15.8	B
	SB-LTR(DEF L)	0.43	19.1	B	0.45	21.1	C	0.55	21.3	C	0.43	18.9	B
	SB-TR				0.46	20.8	C						
Neptune Avenue (E-W) Ocean Parkway (N-S)	EB-L	0.79	53.6	D	0.82	53.3	D	0.55	31.8	C	0.69	44.9	D
	EB-T	0.44	27.3	C	0.42	24.8	C	0.50	29.7	C	0.38	25.8	C
	EB-R	0.02	20.7	C	0.08	19.6	B	0.08	22.6	C	0.09	21.6	C
	WB-LTR	0.57	39.8	D	0.65	42.0	D	0.56	39.5	D	0.51	36.3	D
	NB-L	0.10	22.8	C	0.19	29.4	C	0.27	32.3	C	0.13	27.2	C
	NB-TR	0.37	31.2	C	0.45	32.4	C	0.58	34.7	C	0.50	33.3	C
	SB-L	0.37	25.4	C	0.51	34.9	C	0.53	37.7	D	0.36	28.2	C
	SB-TR	0.58	34.9	C	0.67	37.1	D	0.79	41.0	D	0.73	38.6	D
	EB-LTR	0.15	28.3	C	0.51	26.0	C	0.49	40.6	D	0.47	25.0	C
Neptune Avenue (E-W) Coney Island Avenue (N-S)	EB-TR	0.76	45.0	D	0.63	26.0	C	0.82	46.8	D	0.63	25.9	C
	WB-L	0.48	39.3	D	0.37	22.6	C	0.63	50.6	D	0.48	25.7	C
	WB-TR	0.64	38.5	D	0.57	24.5	C	0.68	40.3	D	0.58	24.3	C
	NB-LTR	0.30	14.2	B	0.50	17.6	B	0.39	15.5	B	0.46	16.9	B
	SB-LTR	0.44	16.1	B	0.71	23.0	C	0.54	18.0	B	0.56	18.8	B
	EB-LTR	0.96	53.2	D	0.54	20.2	C	0.61	27.2	C	0.6	21.2	C
Emmons Avenue (E-W) Nostrand Avenue (N-S)	WB-TR	0.27	16.3	B	0.36	17.2	B	0.36	21.8	C	0.33	16.9	B
	SB-L	0.27	25.9	C	0.18	15.7	B	0.22	20.3	C	0.18	15.7	B
	SB-R	0.3	26.7	C	0.24	16.6	B	0.23	20.6	C	0.23	16.4	B
	EB-LTR												

*Shaded is where changes applied.

APPENDIX 2: TRAFFIC ANALYSIS

Table 5

INTERSECTION AND APPROACH	AM				MD/SAT				PM			
	Phase	Existing Green	Recommended Green	Y/R	Phase	Phase	Recommended Green	Y/R	Phase	Phase	Recommended Green	Y/R
Shore Pkwy South Service Rd/Bay Pkwy												
EB LTR/	1	37	42	5	1	28	28	5	1	37	42	5
NB Ped/	2	8	8	~	2	8	8	~	2	8	8	~
NB TR /SB LT	3	34	31	5	3	24	22	5	3	34	31	5
SB LT	4	26	24	5	4	15	17	5	4	26	24	5
<i>Cycle Length</i>		120				90				120		
Neptune Avenue/West 31st Street												
EB LT /WB T	1	48.6	63	5.4	1	48.6	48.6	5.4	1	48.6	48.6	5.4
NB LTR /SB LR	2	30.6	47	5.4	2	30.6	30.6	5.4	2	30.6	30.6	5.4
<i>Cycle Length</i>		90				90				90		
Neptune Avenue/Cropsey Avenue												
EB LTR/SB R	1	15.3	15.3	5.4	1	15.3	15.3	5.4	1	15.3	15.3	5.4
EB LTR /WB LTR	2	28.8	24.8	5.4	2	28.8	24.8	5.4	2	28.8	24.8	5.4
NB LTR /SB LTR	3	29.7	33.7	5.4	3	29.7	33.7	5.4	3	29.7	33.7	5.4
<i>Cycle Length</i>		90				90				90		
Neptune Avenue/Ocean Parkway												
EB LTR/	1	9	11	5	1	8	13	5	1	9	9	5
EB LTR /WB LTR	2	34	34	6	2	37	35	6	2	34	34	6
NB LTR /SB LTR	3	42	40	5	3	40	40	5	3	40	40	5
NB L /SB L	4	14	14	5	4	14	11	5	4	16	16	5
<i>Cycle Length</i>		120				120				120		
Neptune Avenue/Coney Island Avenue												
EB LTR /WB LTR	1	34	42	5	1	37	37	5	1	34	42	5
NB LTR /SB LTR	2	76	68	5	2	43	43	5	2	76	68	5
<i>Cycle Length</i>		120				90				120		

Shaded areas indicate change in signal timing.

APPENDIX 3: PARKING ANALYSIS

Introduction

Recommendations developed in Links 1 and 7 propose to eliminate on-street parking and reassign road space as Class 2 bicycle lanes. In Link 1 recommended options to stripe a bi-directional bicycle facility on-street or build a shared-use sidewalk eliminate parking on the south side of Shore Road South. In Link 7 proposed options to stripe bicycle lanes along either the curb or the median remove a lane of parking on Emmons Avenue. Parking capacity and utilization studies were conducted to assess the potential impacts of the recommended actions.

Data Collection

On-street parking conditions were examined on Shore Road South between Bay Parkway and 26th Street, and on Emmons Avenue between Ocean Avenue and Knapp Street. Extensive field work, route reconnaissance, and data collection was completed over the course of several days in Summer 2003 in order to capture the peak seasonal traffic generated by activities on the Coney Island waterfront. For both street segments the study compiled data on curbside parking regulations, the number of available parking spaces, including metered spaces, on each side of the street, and the number of cars parked legally or illegally during the weekday AM, MD, and PM peak hour.

APPENDIX 3: PARKING ANALYSIS

On-Street Parking

Link 1: Existing Conditions

Off-street parking lots at the mall adjacent to the Shore Parkway South Service Road and Bay Parkway provide over 1,200 parking spaces, thereby satisfying most of the parking demand in the area. Shore Road South between Bay Parkway and 26th Street has a total of 118 on-street parallel parking spaces, 101 spaces along the north curb and 17 metered spaces along the south curb. During the AM, MD, and PM peak periods, the occupancy rates along the north curb were 65, 63, and 58 percent, respectively. Combining the 17 metered parking spaces and illegal parking on the south curb, the occupancy rates were 68, 70, and 61 percent during the morning, midday, and evening peak periods, respectively. The capacity and utilization rates are shown in Table 1.

Link 1: Future Conditions

The elimination of the 17 parking spaces from the south curb would result in occupancy rates along the north curb of 80, 83, and 72 percent in the AM, Midday, and PM peak hours, respectively. The 101 north-side parking spaces would be sufficient to meet the existing demand, although the removal of metered spaces would result in a loss of revenue.

Link 7: Existing Conditions

Emmons Avenue between Ocean Avenue and Knapp Street provides on-street parking along both the north and south curbs and along either side of the raised/painted median. On eastbound Emmons Avenue there are 26 parallel parking spaces along the south curb and 221 angled parking spaces along the center median; on westbound Emmons Avenue there are 86 parallel parking spaces along the north curb and 150 parallel parking spaces along the median. The total utilization rates during the AM, MD, and PM peak periods (Tables 2 and 3) are 75, 93, and 63 percent, respectively.

Link 7: Future Conditions

The proposed removal of the parallel parking lane along the westbound center median for approximately 10 blocks would result in a loss of 150 spaces, reducing the overall on-street parking supply from 483 to 333 spaces. During the AM peak, 29 spaces, or 9 percent of the existing demand, would not be met. During the MD peak, 118 spaces, or 35 percent of the demand, would not be met. During the PM peak, 92 percent of the available spaces would be occupied, resulting in an excess supply of 28 spaces. The elimination of a parking lane to provide on-street bicycle lanes is not supported by the analysis.

APPENDIX 3: PARKING ANALYSIS

Table 1

Link 1: Existing Parking Capacity and Utilization

Northside - Curbside									Southside - Curbside								
	Signage*	Available Parking Spaces	Number of Spaces Occupied							Signage*	Available Parking Spaces	Number of Spaces Occupied					
			AM		MD		PM					AM		MD		PM	
			Legal	Illegal	Legal	Illegal	Legal	Illegal				Legal	Illegal	Legal	Illegal	Legal	Illegal
Bay Parkway									Bay Parkway								
<i>between</i>	3	0	0	0	0	0	0	0	<i>between</i>	2, 3, 4	0	0	0	0	0	0	0
Shore Parkway On Ramp									Shore Parkway On Ramp								
<i>between</i>	8	22	19	0	16	0	9	0	<i>between</i>	2	0	0	0	0	0	0	0
East end of Mall									East End of Mall								
<i>between</i>	8	20	20	0	16	0	18	0	<i>between</i>	4, 6, 7	9 Meters	2	3	5	3	8	0
After New York Sport Club									Atlantic Express								
<i>between</i>	8	22	18	0	21	0	20	0	<i>between</i>	4, 6, 7	8 Meters	1	0	4	0	4	1
Bayside Oil									Bayside Oil								
<i>between</i>	3, 8	10	6	0	3	0	7	0	<i>between</i>	2	0	0	0	0	0	0	0
25th Avenue									25th Avenue								
<i>between</i>	8	27	3	0	8	0	5	0	<i>between</i>	1	0	0	1	0	0	0	0
Bay 41st Street									Bay 41st Street								
<i>between</i>	3	0	0	0	0	0	0	0	<i>between</i>	4, 5	0	0	7	0	7	0	0
26th Avenue									26th Avenue								

* Posted Signs Chart
 1 = No Signage
 2 = No Parking Anytime
 3 = No Standing Anytime
 4 = Bus Stop
 5 = Bus No Parking Sign
 6 = No Parking 8:30-9am Sat
 7 = 3 Hour Parking 9am-9pm
 8 = No Parking 9-10:30am Sat

APPENDIX 3: PARKING ANALYSIS

Table 2

Link 7 Existing Parking Capacity and Utilization ~ Eastbound																	
Eastbound																	
Northside - Median									Southside - Curbside								
Street	Signage*	Available Parking Spaces	Number of Spaces Occupied						Street	Signage*	Available Parking Spaces	Number of Spaces Occupied					
			AM		MD		PM					AM		MD		PM	
			Legal	Illegal	Legal	Illegal	Legal	Illegal				Legal	Illegal	Legal	Illegal	Legal	Illegal
Ocean Avenue									Ocean Avenue								
<i>between</i>	N	19**	1	0	14	0	5	0	<i>between</i>	N	0	0	0	0	0	0	0
E 21 Street									E 21 Street								
<i>between</i>	N	54	39	0	54	2	23	0	<i>between</i>	N	0	0	0	0	0	0	0
Dooley Street									Dooley Street								
<i>between</i>	N	17	17	0	17	1	0	0	<i>between</i>	N	0	0	0	0	0	0	0
E. 23 Street									E. 23 Street								
<i>between</i>	N	2	2	0	2	0	0	0	<i>between</i>	N	0	0	0	0	0	0	0
Bedford Avenue									Bedford Avenue								
<i>between</i>	N	18	18	0	18	0	3	0	<i>between</i>	N	0	0	0	0	0	0	0
E. 26 Street									E. 26 Street								
<i>between</i>	N	20	16	0	17	0	4	0	<i>between</i>	N	0	0	2	0	0	0	0
E. 27 Street									E. 27 Street								
<i>between</i>	N	13	9	0	12	0	12	0	<i>between</i>	N	0	0	1	0	0	0	0
E. 28 Street									E. 28 Street								
<i>between</i>	N	18	3	0	9	0	6	0	<i>between</i>	N	0	0	0	0	0	0	0
E. 29 Street									E. 29 Street								
<i>between</i>	N	18	6	0	9	0	8	0	<i>between</i>	N	0	0	0	0	1	0	0
Nostrand Avenue									Nostrand Avenue								
<i>between</i>	N	30	21	0	26	0	27	0	<i>between</i>	N	20	15	1	20	1	18	0
Batchelder Street									Batchelder Street								
<i>between</i>	N	6	3	0	1	0	2	0	<i>between</i>	N	5	4	1	5	2	4	0
Ford Street									Ford Street								
<i>between</i>	N	6	6	0	3	0	2	0	<i>between</i>	N	0	0	0	0	0	0	0
Coyle Street									Coyle Street								
<i>between</i>	N	8	6	0	5	0	4	0	<i>between</i>	N	1	1	2	0	2	0	2
Bragg Street									Bragg Street								
<i>between</i>	N	11	11	0	7	0	9	0	<i>between</i>	N	0	0	0	0	0	0	0
Brigham Street									Brigham Street								
<i>between</i>	N	0	0	0	0	0	0	0	<i>between</i>	N	0	0	0	0	0	0	0
Knapp Street									Knapp Street								

* N = No Signage
 ** Parking Meter Spaces

APPENDIX 3: PARKING ANALYSIS

Table 3

Link 7 Existing Parking Capacity and Utilization ~ Westbound

Westbound																	
Southside - Median									Northside - Curbside								
Street	Signage*	Available Parking Spaces	Number of Spaces Occupied						Street	Signage*	Available Parking Spaces	Number of Spaces Occupied					
			AM		MD		PM					AM		MD		PM	
			Legal	Illegal	Legal	Illegal	Legal	Illegal				Legal	Illegal	Legal	Illegal	Legal	Illegal
Knapp Street																	
<i>between</i>	N	9	7	0	7	0	9	0	Knapp Street	N	7	5	0	6	0	7	1
Brigham Street									Brigham Street								
<i>between</i>	N	10	9	0	8	0	9	0	<i>between</i>	N	6	4	0	4	0	5	0
Bragg Street									Bragg Street								
<i>between</i>	N	9	9	0	7	0	5	0	<i>between</i>	N	6	4	0	6	0	3	0
Coyle Street									Coyle Street								
<i>between</i>	N	7	7	0	6	0	5	0	<i>between</i>	N	3	3	0	3	0	1	0
Ford Street									Ford Street								
<i>between</i>	N	6	5	0	4	0	1	0	<i>between</i>	N	5	2	1	2	0	2	0
Batchelder Street									Batchelder Street								
<i>between</i>	N	14	10	0	13	1	9	0	<i>between</i>	N	5	4	0	4	1	4	0
Brown Street									Brown Street								
<i>between</i>	N	0	0	0	0	0	0	0	<i>between</i>	N	1	0	0	0	0	1	0
Haring Street									Haring Street								
<i>between</i>	N	13	10	0	11	0	11	0	<i>between</i>	N	12	6	0	8	0	9	1
Nostrand Avenue									Nostrand Avenue								
<i>between</i>	N	8	4	0	5	0	8	0	<i>between</i>	N	8	1	0	3	0	5	0
E. 29 Street									E. 29 Street								
<i>between</i>	N	8	5	0	5	0	7	0	<i>between</i>	N	8	0	0	4	0	0	0
E. 28 Street									E. 28 Street								
<i>between</i>	N	7	6	1	6	0	4	0	<i>between</i>	N	0	0	0	0	1	0	1
E. 27 Street									E. 27 Street								
<i>between</i>	N	13	7	0	9	0	2	0	<i>between</i>	N	12	4	0	8	0	6	0
E. 26 Street									E. 26 Street								
<i>between</i>	N	10	8	0	8	0	6	0	<i>between</i>	N	4	1	0	3	0	3	0
Bedford Street									Bedford Street								
<i>between</i>	N	1	1	1	1	0	0	0	<i>between</i>	N	2	2	0	2	0	1	0
E. 23 Street									E. 23 Street								
<i>between</i>	N	8	8	0	8	0	7	0	<i>between</i>	N	7	5	0	7	1	4	0
Dooley Street									Dooley Street								
<i>between</i>	N	27	26	0	27	1	18	0	<i>between</i>	N	21**	6	0	20	2	10	0
E. 21 Street									E. 21 Street								
<i>between</i>	N	10**	1	0	4	0	4	0	<i>between</i>	N	8**	4	0	5	2	7	0
Ocean Street									Ocean Street								

* N = No Signage
 ** Parking Meter Spaces

APPENDIX 4: ACCIDENT ANALYSIS

Introduction

An analysis of study area accident data for the last three available years (1998-2000) was conducted at selected intersections to further understand existing conditions along the proposed route. All data was obtained from the New York State Department of Motor Vehicles (NYSDMV) accident database, which summarizes information from local New York City Police Department (NYPD) accident reports. The data includes both reportable and non-reportable accidents. The NYSDMV designates motor vehicle accidents as reportable if they result in physical damages of \$1,000 or more or if there is an injury or fatality. Accidents resulting in less monetary damage or no injuries are designated as non-reportable (which does not mean that a police report was not filed.) Pedestrian and bicycle accidents are reportable by their very nature.

Accidents

Total, reportable, and pedestrian/bicycle accidents were examined at the same seven intersections subject to traffic analysis, as shown in Table 9. Overall, there were not high numbers of accidents along the corridor. The total accidents at all seven intersections averaged less than 25 per year; total reportable accidents averaged less than 13 per year. Neptune Avenue at Ocean Parkway, at Coney Island Avenue, and at Cropsey Avenue/West 17th Street had the highest numbers of total and reportable accidents along the route, but even these three intersections had on average fewer than 22 reportable accidents per year. Neptune Avenue and West 31st Street had the fewest reportable accidents of the seven intersections analyzed, but also had the only fatality recorded during the three-year period.

Between 1998 and 2000 there were only 30 total pedestrian accidents, or an average of 10 per year, at all seven intersections combined. Neptune Avenue at Ocean Parkway (seven) and Coney Island Avenue (seven), and Shore Road South at Bay Parkway (six), had the most pedestrian accidents over three years.

Table 1

Intersection	Total	Reportable	Fatalities	Pedestrians	Cyclists
Bay Parkway and Shore Road South	59	21	0	6	1
Neptune Avenue and West 31th Street	23	15	1	2	2
Neptune Avenue and Cropsey Avenue/West 17th Street	85	42	0	3	0
Neptune Avenue and Stillwell Avenue	43	19	0	5	0
Neptune Avenue and Coney Island Avenue	112	58	0	7	4
Neptune Avenue and Ocean Parkway	169	95	0	7	0
Emmons Avenue and Nostrand Avenue	30	13	0	0	1

APPENDIX 5: TECHNICAL ADVISORY COMMITTEE

The following is a list of individuals and organizations that participated or were invited to participate in this project.

Elected Officials

The Honorable Marty Markowitz, President of the Borough of Brooklyn
 The Honorable Lewis A. Fidler, City Council Member, District 46
 The Honorable Domenic Recchia, City Council Member, District 47
 The Honorable Michael Nelson, City Council Member, District 48
 The Honorable Carl Kruger, State Senator, District 21
 The Honorable Seymour Lachman, State Senator, District 22
 The Honorable Helene E. Weinstein, State Assembly, District 41
 The Honorable Steven Cymbrowitz, State Assembly, District 45
 The Honorable Adele Cohen, State Assembly, District 46
 The Honorable William Colton, State Assembly, District 47

Elizabeth Ernish, Borough President Markowitz's Office
 Rodney Knight, Councilmen Recchia's Office
 Marc Rivlin, State Senator Lachman's Office
 Roslyn Sokol, State Assembly Cymbrowitz's Office
 Michael Treybich, State Assembly Cohen's Office

New York City and State Agencies

Jennifer Hoppa, New York City Department of Parks and Recreation
 Joshua Laird, New York City Department of Parks and Recreation
 Tweeps Phillips, New York City Department of Parks and Recreation
 Julius Spiegel, New York City Department of Parks and Recreation
 Lori Ardito, New York City Department of Transportation
 Holly Haff, New York City Department of Transportation
 Dalila Hall, New York City Department of Transportation
 Dan Orlando, New York City Department of Transportation
 Michael Primeggia, New York City Department of Transportation
 Naim Rasheed, New York City Department of Transportation
 Carren Simpson, New York City Department of Transportation
 Anne Marie Turner, New York City Department of Transportation
 Milorad Ubiparip, New York City Department of Transportation
 Andrew Vesselinovitch, New York City Department of Transportation
 Aizaz Ahmed, New York Metropolitan Transportation Council
 Gerard J. Bogacz, New York Metropolitan Transportation Council
 Guy La Monaca, New York State Department of Transportation
 Roger Weld, New York State Department of Transportation
 Community Affairs, Brooklyn South Precinct, NYC Police Dept. Precinct 60
 Community Affairs, Brooklyn South Precinct, NYC Police Dept. Precinct 61
 Community Affairs, Brooklyn South Precinct, NYC Police Dept. Precinct 62

Community Boards

Howard Feuer, Brooklyn Community Board 11
 William R. Guarinello, Brooklyn Community Board 11
 Corrado Manfredi, Brooklyn Community Board 11
 Brian Gotlieb, Brooklyn Community Board 13
 Jeannette LoSciuto, Brooklyn Community Board 13
 Chuck Reichenthal, Brooklyn Community Board 13
 Ida Sanoff, Brooklyn Community Board 13
 Barbara Teitelbaum, Brooklyn Community Board 13
 Paula Lupka, Brooklyn Community Board 15
 John E. Nikas, Brooklyn Community Board 15

Civic Groups, Associations, Organizations and Citizens

Maggie Bowman, Transportation Alternatives
 Noah Budnick, Transportation Alternatives
 John Comer, Community School District 22
 Edward DeFreitas, Five Borough Bike Club
 Diana Gavales, Transportation Alternatives
 Heather Jacksy, Transportation Alternatives
 Gary Katz, Bicyclist
 Dave Lutz, Neighborhood Open Space Coalition
 Byron McClenney, Kingsborough Community College
 New York Cycle Club
 Al Normandia, Sizzling Bicycles
 Nostrand Avenue Merchants' Association
 Mario Novello, Bath Beach Cycles
 Brian Palmer, Coney Island Hospital
 Ed Pino, Five Borough Bike Club
 Mary Powell, Madison Marine Civic Association
 Gil Retrey, Kingsborough Community College
 Jeff Sandgrund, Partnership For Parks
 Pat Singer, Brighton Neighborhood Association
 Louis Spina, Kings Bay Youth Organization
 Ellen Susnow, Greater Sheepshead Bay Development Corporation
 Pam Tice, Bike New York
 Ethel Tucker, Community School District 21
 Edna Wildman, Kings Bay YM/YWHA

CREDITS

New York City Department of City Planning

Amanda M. Burden, AICP, Director
Richard Barth, Executive Director
Sandy Hornick, Deputy Executive Director for Strategic Planning
Barbara Weisberg, Assistant Executive Director of Planning Coordination
Elizabeth Mackintosh, Director of Studies
Regina Myer, Director, Brooklyn Office
Winston von Engel, Deputy Director, Brooklyn Office

New York City Department of City Planning, Transportation Division

Jack Schmidt, Director
Kevin Olinger, Deputy Director
Scott Wise, Team Leader, Pedestrian, Bicycle, and Greenway Projects
Stephen Johnson, Project Manager
Indradeep Chakrabarty, Urban Designer
Susan Lim, Highway Transportation Specialist
Caroline Forger, former City Planning Technician

The following Transportation Division staff members participated in fieldwork that contributed to the completion of this document:

Hassan Adekoya, Xiomara Aguilera, Cornelius Armentrout, Karen Blatt, Jennifer Dickson, Lise Dorestant, Kate Dyson, Emilio Feliz, Laura Fink, Miriam Herzfeld, Rex Hodgson, Karen Johnson, Emily Karpel, Angela Kelly, Andre McGlashen, John Nassis, Olga Olovyannikov, Alan Ripps, Nora Santiago, Erik Seims, Hisa Tanaka and Britta Van Aartsen.